

Ground Response to:

Foundation and Ground Improvement Installation

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HOME / CONFERENCES

school of Engineering Civil, Environmental & Architectural Engineering

1989 Toyota Land Cruiser







NUMBER ONE SONGS:

Beautiful Crazy - Luke Combs

Chicken Fried – Zac Brown Band Toes – Zac Brown Band Whatever It Is – Zac Brown Band Highway 20 Ride – Zac Brown Band As She's Walking Away feat. Alan Jackson – Zac Brown Band Knee Deep feat. Jimmy Buffett – Zac Brown Band Colder Weather – Zac Brown Band Keep Me in Mind – Zac Brown Band Goodbye in Her Eyes – Zac Brown Band Sweet Annie – Zac Brown Band Homegrown – Zac Brown Band

CPT Sounding



Siegel, T.C. and McGillivray, A. (2009) Interpreted Residual Load in an ACIP Pile, *Proceedings*, Annual DFI Conference.

Test Pile Details

Results of Strain Monitoring



Interpreted Residual Load versus Depth



toe resistance

Profiles of Interpreted Ultimate Residual Load



<u>Residual Load on Piles</u>

Foothills Parkway Wears Valley TN (near the Great Smoky Mountains)



Luna, R., Dixon, D.T., Kershaw, K.A., and Siegel, T.C. (2015) Monitoring micropile foundations of bridge during construction, IFCEE Conference.



FIG. 3. Footing Plan (Corven Eng., Inc. & Palmer Eng., Inc., 2010)



Luna, R., Dixon, D.T., Kershaw, K.A., and Siegel, T.C. (2015) Monitoring micropile foundations of bridge during construction, IFCEE Conference.



There is no such thing as a stress-free pile.

Ground Response to Auger Rotations on (Dry) Sand





Siegel, T.C. (2012) Testing of augered, cast-in-place piles installed with varying auger rotations, Full-Scale Testing and Foundation Design, GSP 227.

The Influence of Auger Rotations on (Dry) Sand



1 MPa ~ 10 tsf

The Influence of Auger Rotations on (Dry) Sand



The Influence of Auger Rotations on (Dry) Sand

We are all susceptible to groupthink - mode of thinking in which individual members of small cohesive groups tend to accept a viewpoint or conclusion that represents a perceived group consensus

Ground Response to Compaction Grouting



2 <u>COMPACTION GROUTING CROSS SECTION</u> 1.01 NO SCALE

Ground Response to Compaction Grouting



Ground Response to Compaction Grouting



Accept that there are unknown unknowns.





Displacement tooling (Berkel)





Subject site Young Bay Mud Cooper Marl Pore pressure Pore pressure Pore pressure 0 5 5 -5 10 10-10-15 15 15-20 20 -20-25 25 -25 -30 -30 -30-35 -35 -35 40 -40· (#) 45 -fd 50 -0 55 -40 45 · 45 -< 100 psi For comparison 50-50 55 · 55 60 -60-60 65 65-Depth (ft) 65 Depth (ft) 70-70 · 70-75 -75 · 75 -80 -80 -80-85 85 -85 -90 · 90 90-95 · 150 200 250 300 350 400 0 50 100 Pressure (psi) 95 100 -300+ psi 100 105 105 110 -110 115 115 -120 -120 125 -125 130 130 -135 -135 -140 -140 145 145 -200 Pressure (psi) 300 0 100 400 150 200 400 0 Pressure (psi)

Cooper Marl data courtesy of Greg Canivan S&ME, Inc.

400+ psi

Void created by penetration of the displacement element

Very high pore pressures develop and soil becomes essentially incompressible.

Displacement element

"Brittle" Clay Behavior and Drilled Displacement Piles



Below the displacement element, the grout pressures rise quickly as the incremental grout volume exceeds 100% of the hole volume. The ground is "brittle" and fractures. After the ground fractures, the grout pressure drops to the value necessary to drive grout along the fracture.

"Brittle" Clay Behavior and Drilled Displacement Piles



Displacement element

"Brittle" Clay Behavior and Drilled Displacement Piles



What is good in most circumstances may not good in all circumstances

Effect of Discrete Layering



Santa Clara

San Jose







Effect of Discrete Layering



Limitations exist in every analysis.

Ground Response to Retesting CFA Piles



Retesting CFA Piles

16-inch CFA Pile 100 200 300 400 500 600 700 0 Cone resistance qt Soil Behaviour Type 0.00 0 HAND AUGER HAND AUGER 5 -10-10-Clay 0.25 15 15-Clay & silty clay Clay 20-20-Clay & silty clay Displacement (inches) 25-Clay & silty clay 0.50 25-Clay Clay 30-30-Sand & silty sand Silty sand & sandy s Retest @ 114 days 35-35-Clav 0.75 BH 90% = 585 kips Clay 40-40. Clay & silty clay Clay & silty clay £ 45 £ 45 Clay Clay & silty clay Depth Depth 50-1.00 50 Clay Clay & silty clay Test @ 17 days 55-Clay & silty clay 55 Clay & silty clay BH 90% = 360 kips Clay & silty clay 60. 60 Silty sand & sandy si Тор 1.25 Clay & silty clay 65-65-Clay +225 kips Clay & silty clay 70-70-62% Increase 75-75-1.50 80-80-Clay & silty clay 85-85-1.75 90-90-95-95-Silty sand & sandy si Clay & silty clay 100 100 2.00 200 300 40 0 100 0 2 4 6 8 10 12 14 16 18 Tip resistance (tsf) SBT (Robertson, 2010)

Applied Top Load (kips)

Retesting CFA Piles

0

5 -

10-

15-

20-

25-

30

35

40-

45

50

55

60

65

70-

75-

80-

85

90

95-

100

£

Depth

16-inch CFA Pile 200 700 0 100 300 400 500 600 Cone resistance qt Soil Behaviour Type HAND AUGER HAND AUGER 0.25 ᢙ᠆ᢙ᠆ᠬ 10-**-**~~ 15-0.50 Clay & silty clay Clav 20-Clay & silty clay Clay & silty clay 0.75 25-Clay Clay 30-Sand & silty sand Top Displacement (inches) 1.00 Silty sand & sandy s 35-Retest @ 112 days Test @ 15 days Clay 40-Clay & silty clay 1.25 BH 90% = 650 kips BH 90% = 460 kips Clay & silty clay £ 45. Clay Clay & silty clay Depth 1.50 Clay Clay & silty clay 55-Clay & silty clay Clay & silty clay 1.75 Clay & silty clay Silty sand & sandy silt 60-Clay & silty clay 65-Clay 2.00 Clay & silty clay +190 kips 70-Clay 41% Increase 75-2.25 80-Clay & silty clay 2.50 85-90-2.75 95-Silty sand & sandy Clay & silty clay 100-0 100 200 300 400 3.00 0 2 4 6 8 10 12 14 16 18 Tip resistance (tsf) SBT (Robertson, 2010)

Applied Top Load (kips)

Retesting CFA Piles

16-inch CFA Pile 100 200 0 Soil Behaviour Type Cone resistance qt 0.00 * ------0 -HAVID AUGER HAND AUGER 5 -5 0.25 Clay Clay & silty clay 10-10-Clay & silty clay 2 Sand & silty sand 0.50 15-15-Clay & silty clay 20-20-Clay 0.75 25-25-Top Displacement (inches) Clay & silty clay Clay & silty clay 30-30-Clay & silty clay 1.00 Clay Clay & silty clay 35. 35-Clay 1.25 40 40-Clay & silty clay € 45 Depth (ft) 45-1.50 Depth Clay 50 50-55 55-Clay & silty clay 1.75 60 60-Clay 2.00 65 65-Clay & silty clay 70-70-2.25 Clay Clay & silty clay Clay 75-75-Clay & silty clay 80-80-2.50 Silty sand & sandy s 2 Clay & silty clay 85-85-Clay & silty clay 2.75 90· 90-Clay Clay & silty clay 95 95-Clay & silty clay Silty sand & sandy 3.00 100-100-200 300 400 0 100 0 2 4 6 8 10 12 14 16 18 Tip resistance (tsf) SBT (Robertson, 2010)



Applied Top Load (kips)

Ground Response to Retesting CFA Piles



Even for things that we historically think are "**black or white**" – like pile resistance – may not be so.

"It is wrong to faithfully apply the Rules without considering whether the results are correct"

-from Karl Terzaghi: The Engineer as Artist.



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• The Rules are <u>not</u> synonymous with ground behavior.

"It is wrong to faithfully apply the Rules without considering whether the results are correct"

-from Karl Terzaghi: The Engineer as Artist.



- The Rules are <u>not</u> synonymous with ground behavior.
- "Artists" recognize the Rules and their limitations. They do not apply the Rules blindly anddo embrace actual ground behavior when it conflicts with the Rules.

